

3-D Assessment of a T-wall System in New Orleans

Jinoh Won*, Chung R. Song**, Sudarshan Adhikari**,
Alexander H.-D. Cheng** and A. Al-Ostaz**

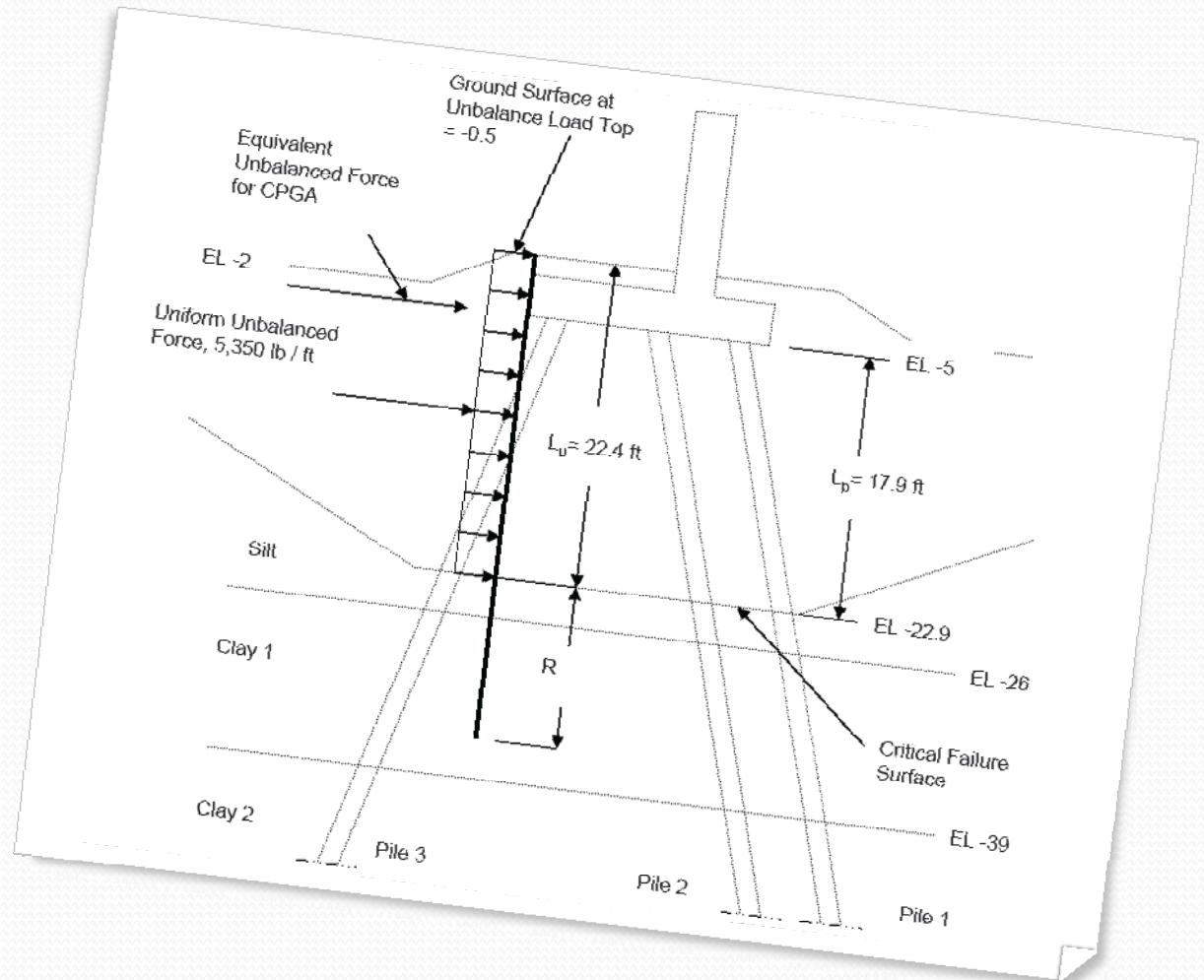
*: Samsung Construction Co. Ltd. Seoul Korea

** : University of Mississippi, University, MS 38655

Motivation

- **T-walls survived Hurricane Katrina.**
- **Are T-walls truly survived and barely survived?**
- **Need sophisticated analysis of T-wall.**
- **Conducted 3-D numerical analysis for T-walls in IHNC, New Orleans.**

Typical T-wall section

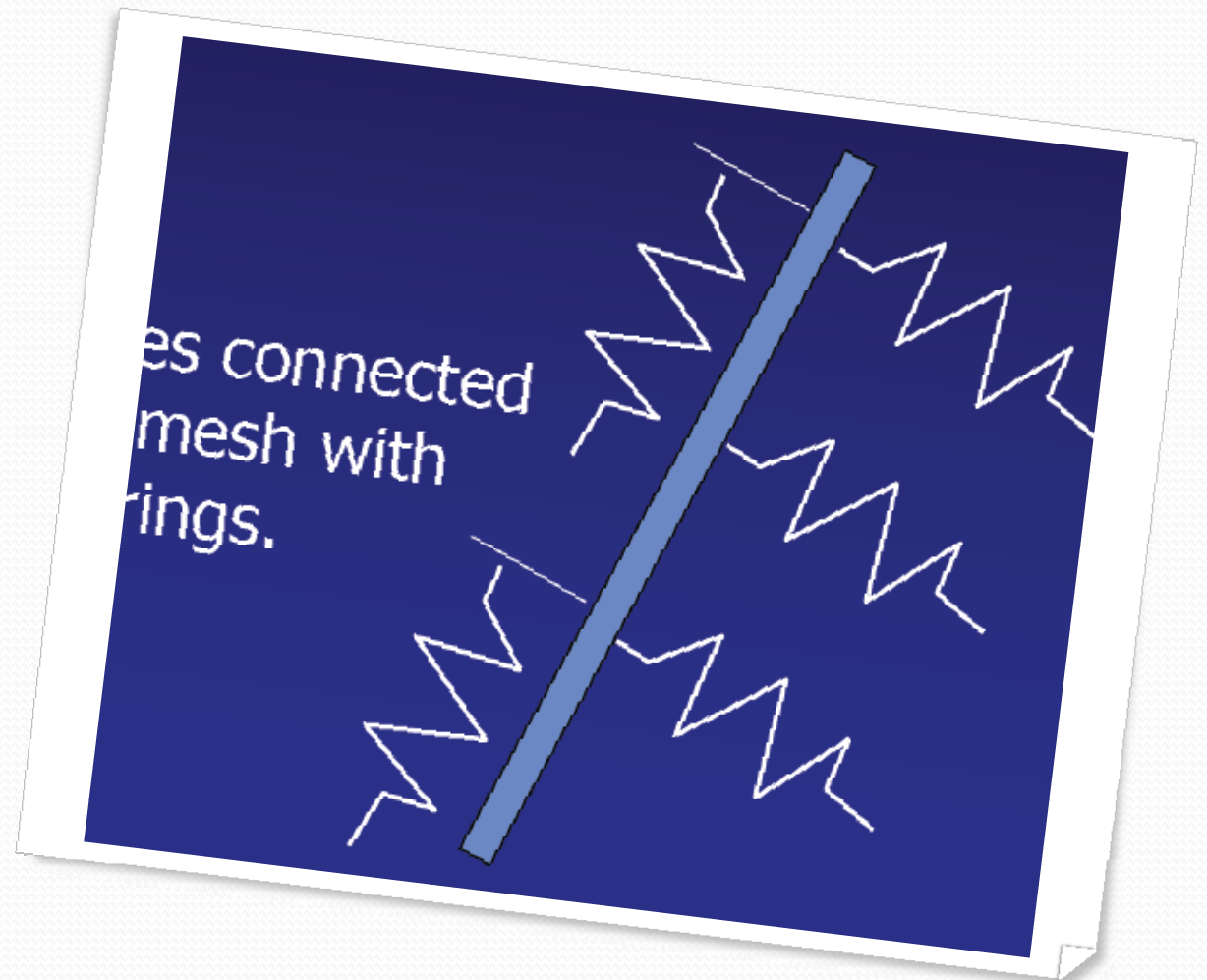


Current Design methods

(Hurricane and Storm Damage Reduction System Design Guidelines, May 2008)

- **Uncoupled analysis**
- **Slope stability** analysis
 - Check Factor of Safety: UTexas4, Slope/W
 - Find “**Unbalanced Load**”
- **Pile Group** analysis
 - Preliminary design with **CPGA** – **check flow through**
 - Calculate pile head loads, cap displacement
 - “**Group 7 Analysis**” of critical cases
- Developed to match with the 2D FLAC analysis results

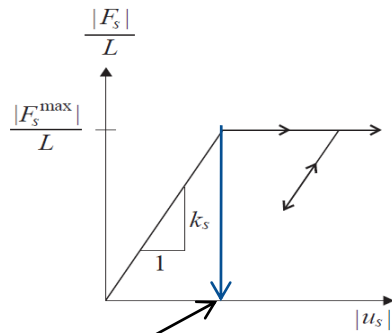
3. Pile structural element in FLAC 3D



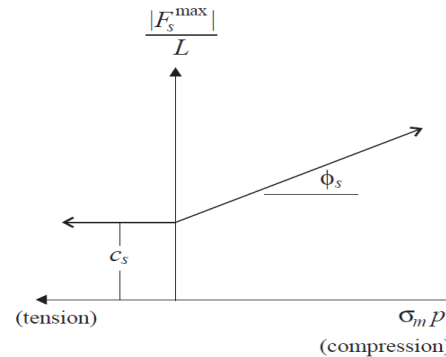
SEs

- We could model everything with zones
- **Zone are poor at modeling bending** (a large number may be required across the thickness)
- **Grid generation** becomes problematic when modeling structures with zones (geometric problems as well as RAM shortage)
- If we are **not interested in small scale local effects** (e.g. detailed stress distribution across an oddly shaped beam section) then **SEs are ideal**.

How to estimate shear coupling spring constants?



(a) shear force/length versus relative shear displacement, u_s



(b) shear-strength criterion

$$\frac{|F_s^{\max}|}{L} = c_s + \sigma_m \cdot \tan(\phi_s) \cdot A_s$$

0.03B for sand
0.01B for clay

Alternative

$$\frac{|F_s^{\max}|}{L} = Q_s = f_s \cdot A_s$$

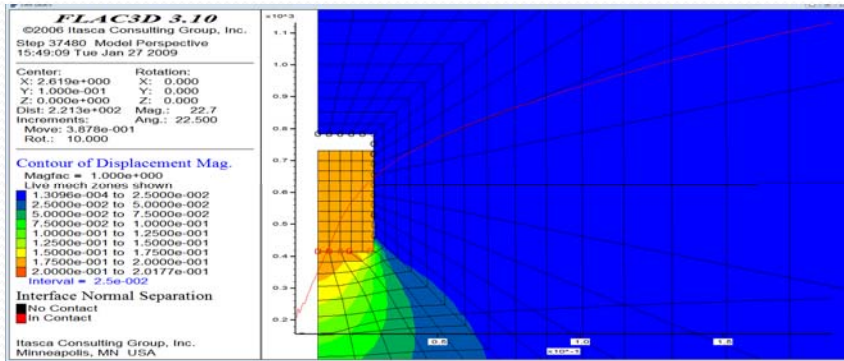
$$f_s = \sigma_h \cdot \mu = (K_o \sigma_v) \tan(\delta) \quad \text{for sand}$$

$$f_s = \alpha \cdot s_u \quad \text{for clay}$$

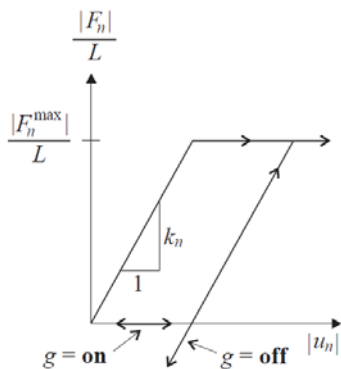
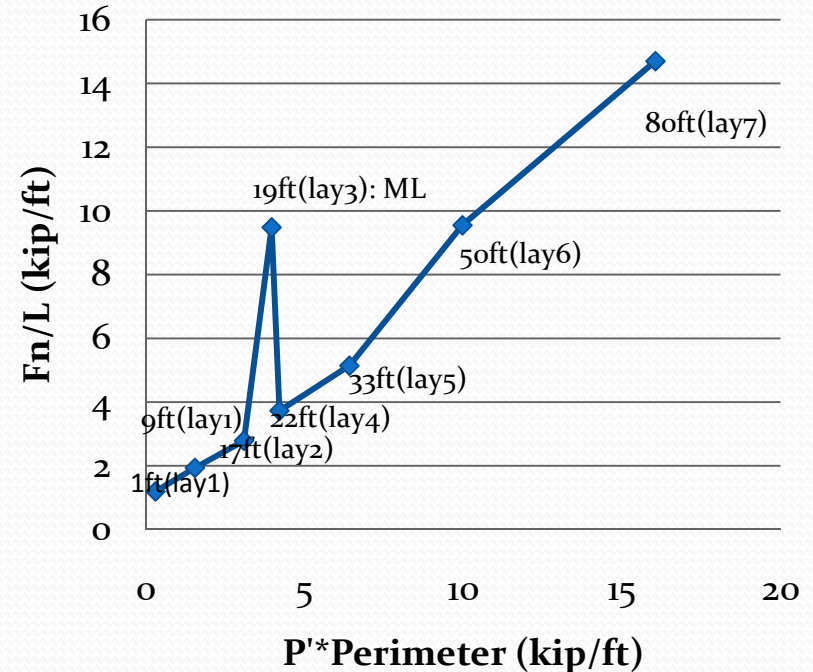
How to estimate normal coupling spring constants?

2D plane strain analysis

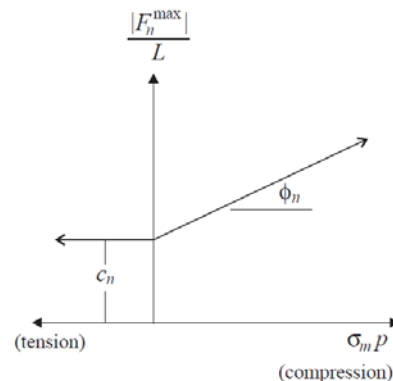
Estimate constants



Fn/L vs P'*peri

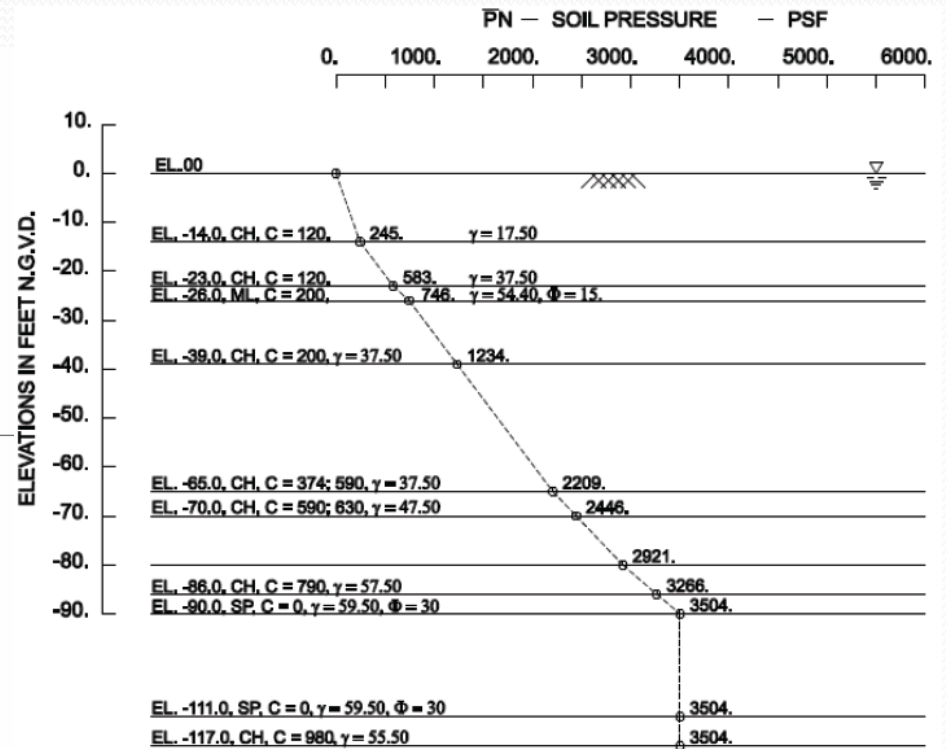
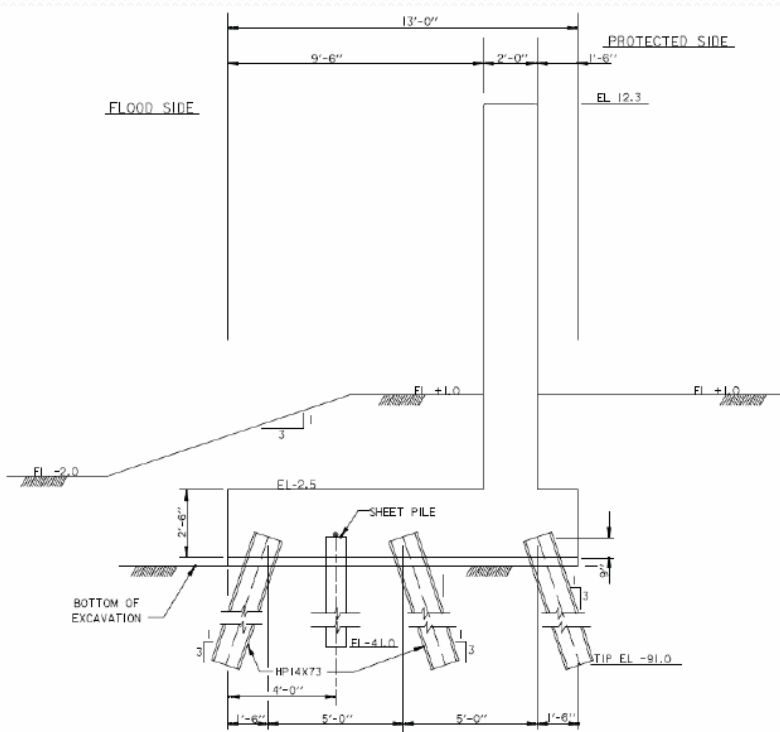


(a) normal force/length versus relative normal displacement, u_n



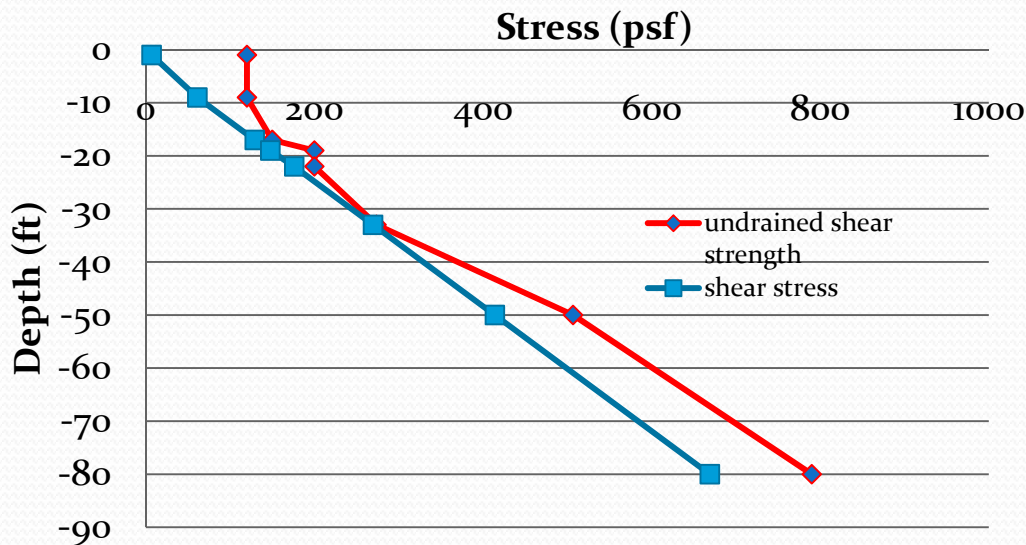
(b) normal-strength criterion

T-wall and ground conditions



Soil properties

Layer #	top ELE	top Depth(ft)	Soil type	c (psf)	c_inc (psf/ft)	ϕ	γ (pcf)	Modulus			
								E (psf)	G/s _v	G (psf)	K (psf)
1	-5	0	CH	120			80		100	1.20E+04	2.96E+05
2	-14	9	CH	150			100		130	1.95E+04	4.81E+05
3	-23	18	ML	200		15	117		130	2.60E+04	6.41E+05
4	-26	21	CH	200			100		130	2.60E+04	6.41E+05
5	-31	26	CH	217	8.1		100		130	2.82E+04	6.96E+05
6	-39	34	CH	374	8.3		100		160	5.98E+04	1.48E+06
7	-70	65	CH	790			100		160	1.26E+05	3.12E+06
8	-86	81	SP			30	115	4.23E+05		1.63E+05	3.53E+05
9	-111	106	CH	980	8.3		115		160	1.57E+05	3.87E+06



Initial shear stress
& undrained shear strength

Pile properties

[For Solid element]

HP 14*73

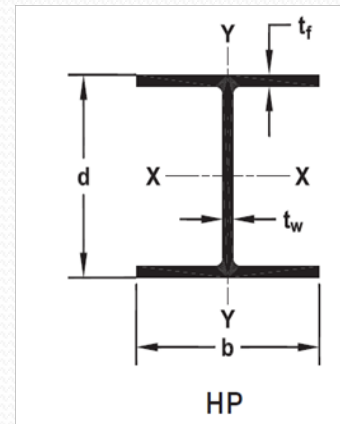
E_steel=	4.18E+09	psf
I _y =	0.035156	ft ⁴
I _{y_solid} =	0.14669453	ft ⁴

Equivalent E

(Same EI)

E_eq=	1.00E+09	psf
nu=	0.3	
K=	4.77E+08	psf
G=	3.85E+08	psf

gamma _t =	139	pcf
density=	4.32	



E_{eq} ?
(EI=const.)

$$b = 1.22 \text{ ft}$$

$$d = 1.12 \text{ ft}$$

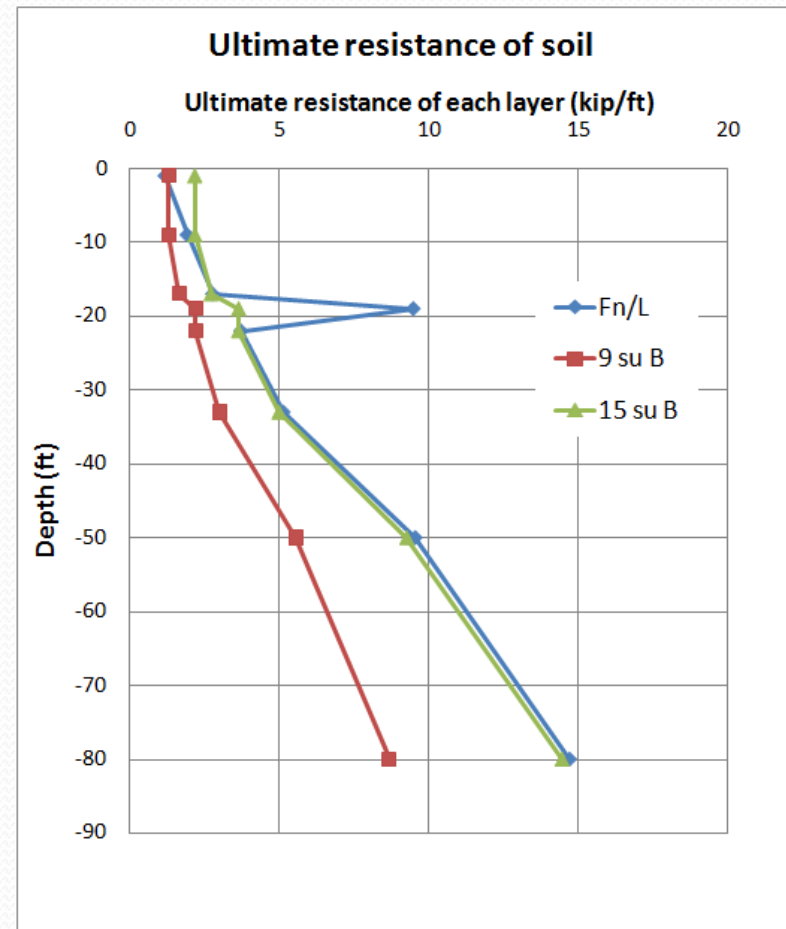
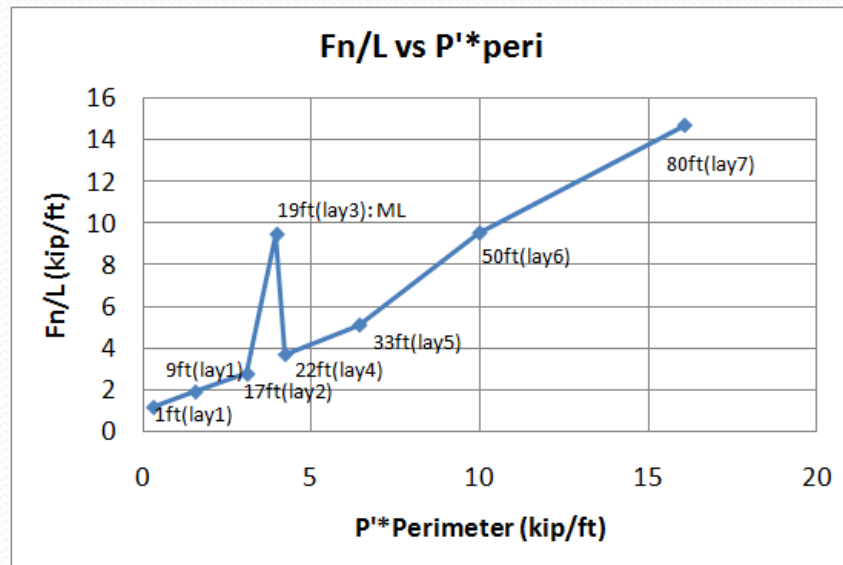
[For Pile element]

(HP14*73)

E (psf)	nu	XCArea	XCJ	Xciy	XCiz	Per
4.18E+09	0.3	0.1486	0.047743	0.01259	0.03516	4.699

Input parameters for pile element

(Normal coupling-spring constants)



* “cs_ncoh”, “cs_nfric” from upper graph

* For rectangular pile in clay,

$$p_{ult} = 15 \cdot s_u \cdot B$$

Task 2B: T-Wall Analysis

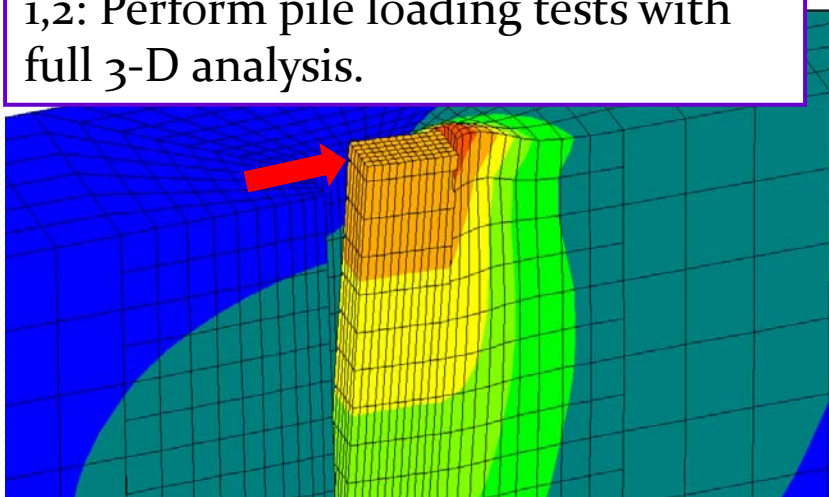
Procedure to Define A Proper Pile Element

1. Perform full 3-D solid element simulation with piles embedded.
2. Apply active force or passive force to piles and compute stresses and strains.
3. From stresses and strains obtain P-y curves for multiple depths.
4. Using P-y curves, find proper soil-pile interaction spring constants and failure loads.
5. Using these soil-pile interaction spring constants and failure loads, perform 3-D simulation with “Pile Elements”.
6. When the results of steps 2 and 5 are almost identical, then the pile element is properly defined.

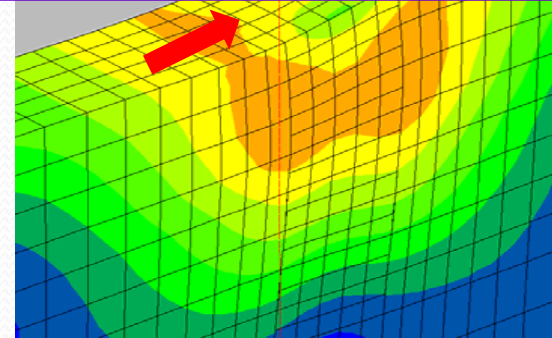
T-Wall Analysis

Procedure to Define A Proper Pile Element

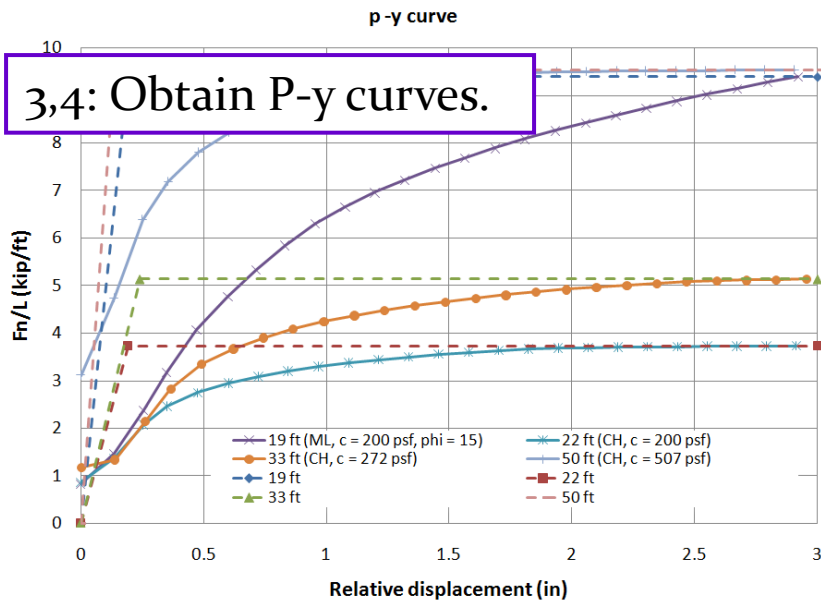
1,2: Perform pile loading tests with full 3-D analysis.



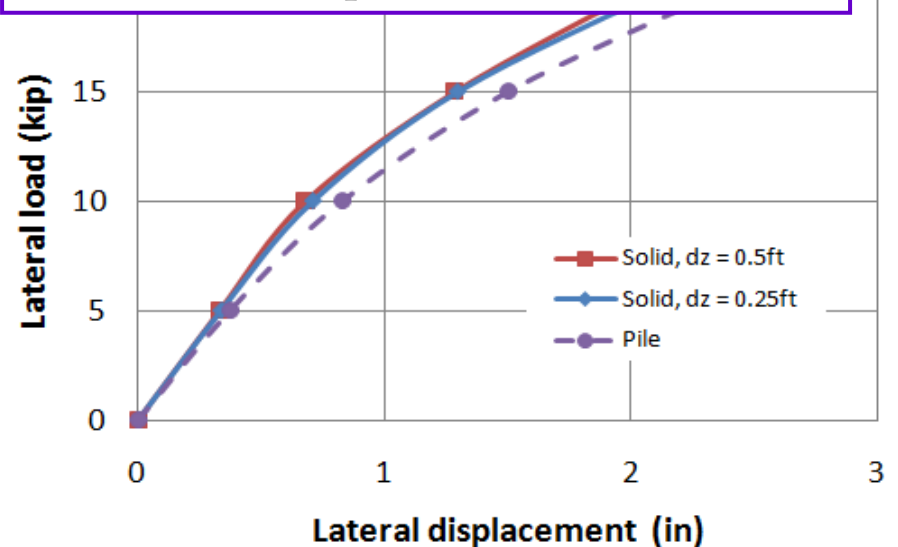
5: Perform pile loading tests with P-y curves and pile element.



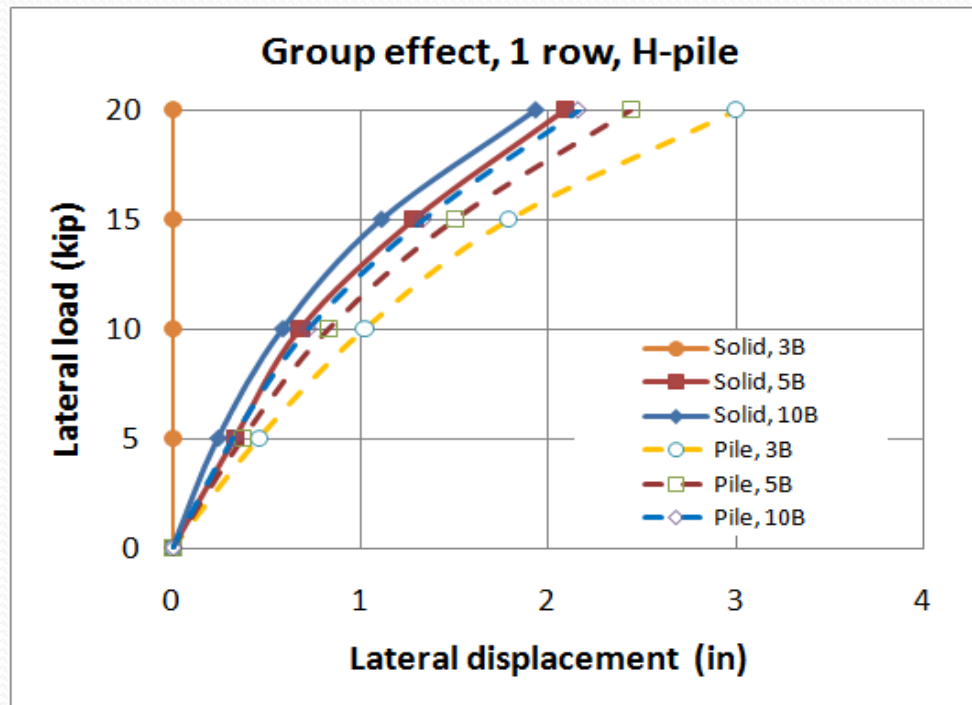
3,4: Obtain P-y curves.



6: Verify P-y curves by comparing full 3-D results and pile element results.

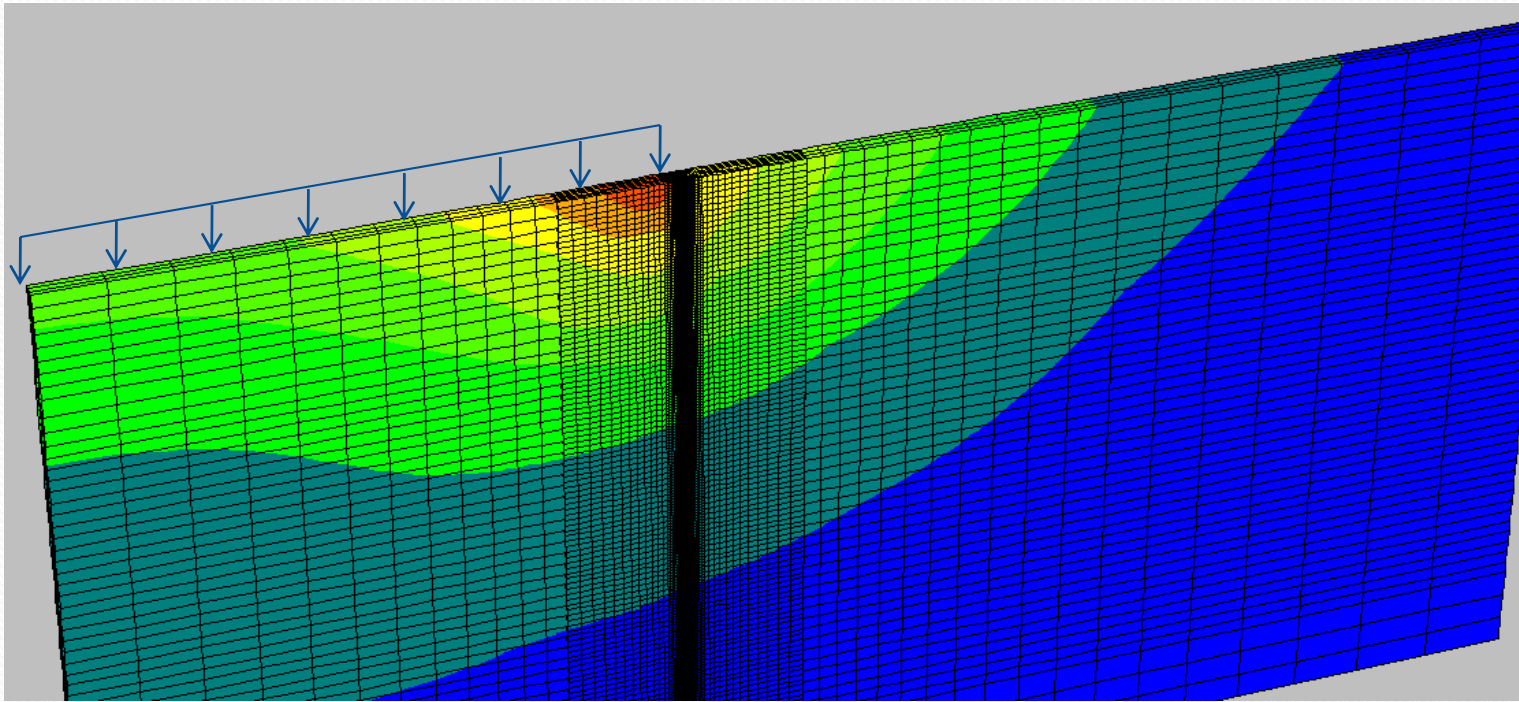


Group effect in active case



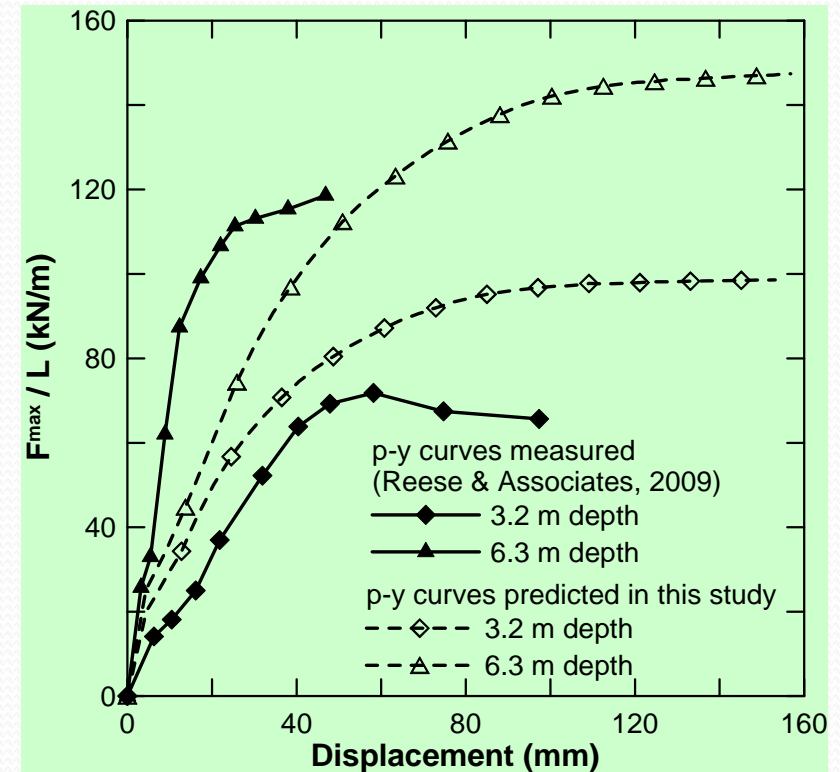
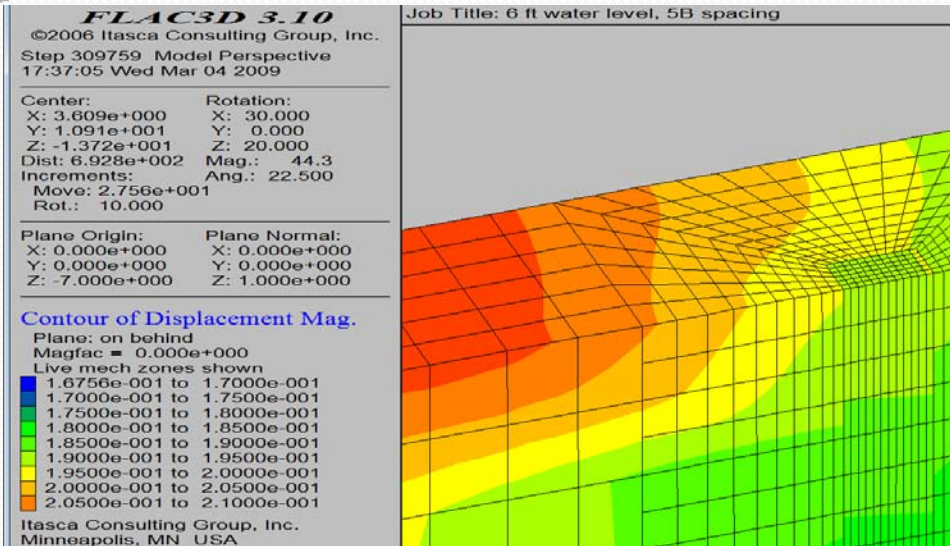
- Displacement increases in closely spaced pile.
- **Group effect is automatically considered in pile 3D analysis.**

3D analysis for passive case

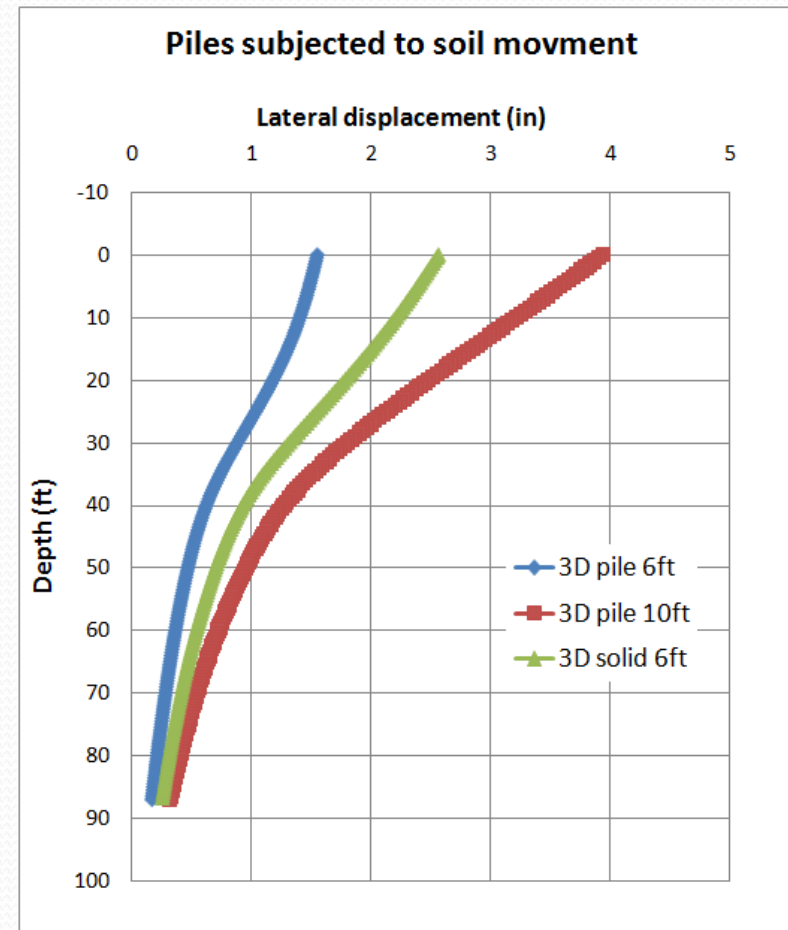
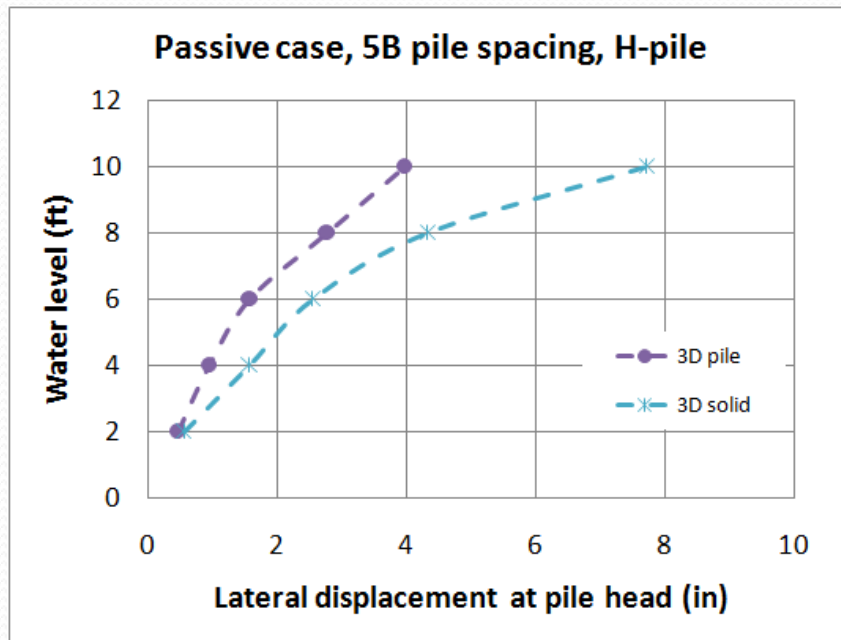


T-Wall Analysis

Calibration using lateral pile load test results

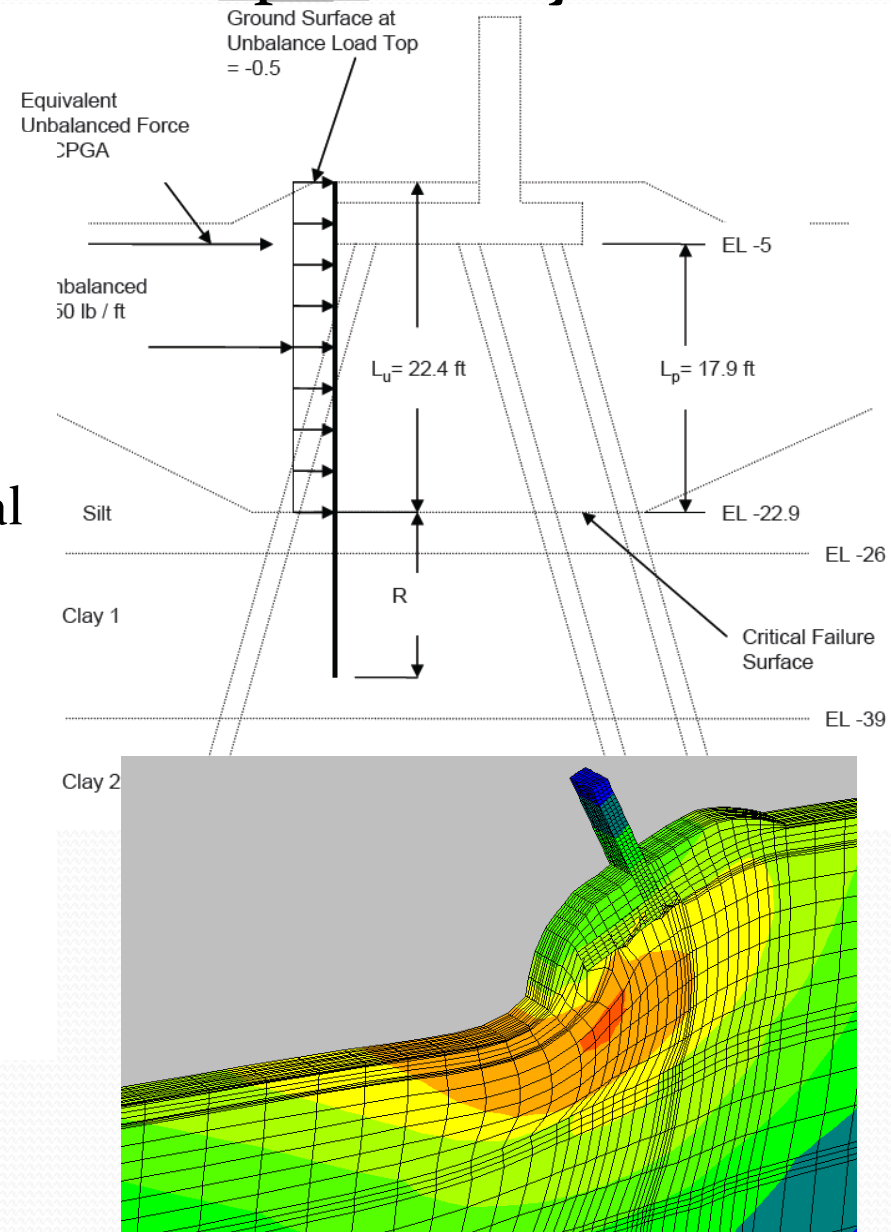


Pile top lateral displacement



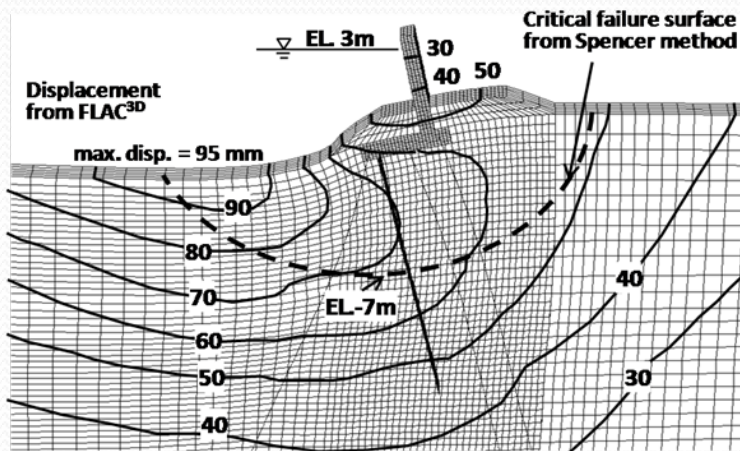
Task 2B: Soil-structure-fluid coupled analysis

- Existing T-wall analysis is mostly 2-D analysis.
- 2-D analysis has limited capability in dealing with the frictional force that exist parallel to the mesh and normal stress and strains along the longitudinal direction.
- By using 3-D FLAC simulation with advanced constitutive models, realistic 3-D simulation of soil-structure interaction for T-wall is conducted.
- Similar 3-D simulation is also applied for **floodwall and structure interfaces**.

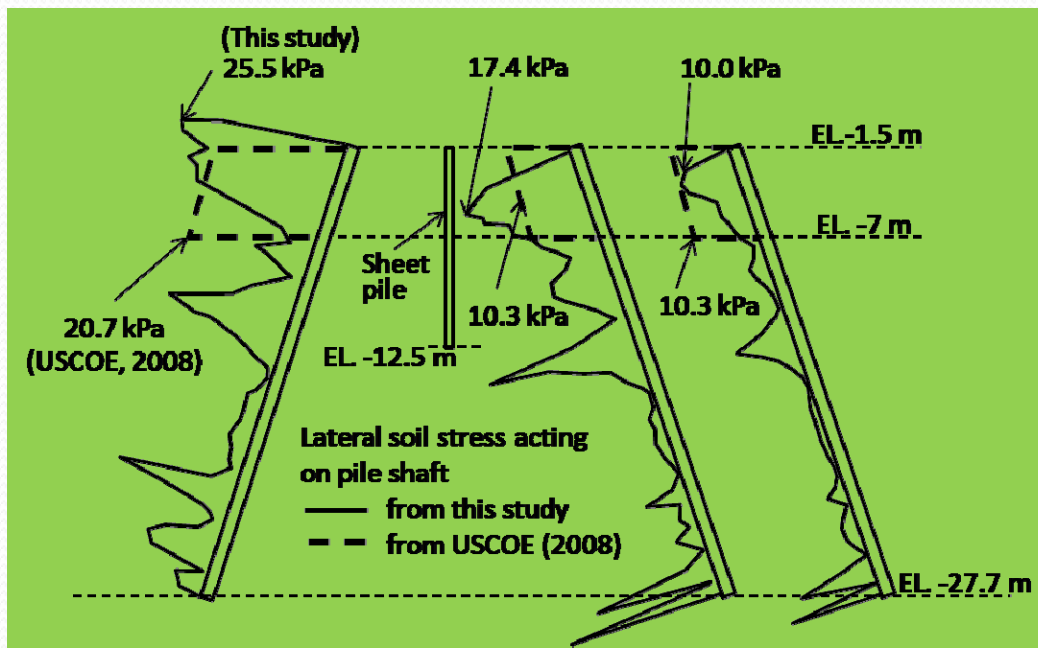
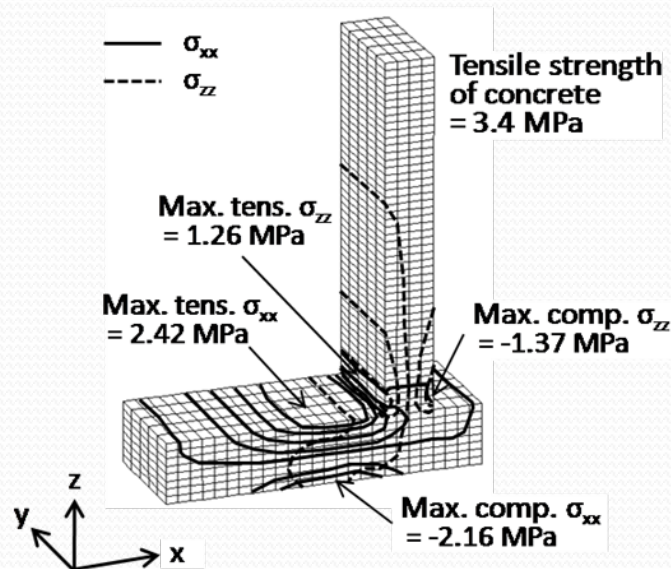
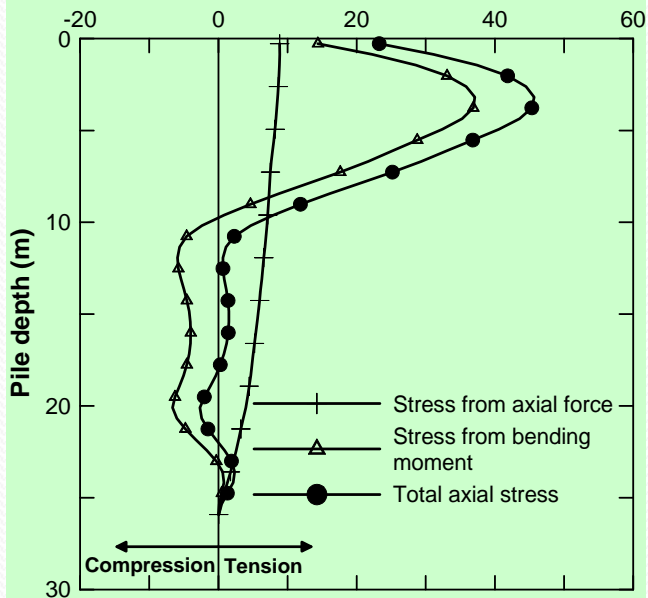


T-Wall Analysis

Behavior of Soils Around Piles



Stress (MN/m²)



Summary

- Pile element provide very good results for active case, including group effect.
- Need validation for pile structural elements for passive case!



Acknowledgement



This project was funded through the [SERRI LEVEE program](#) from the department of Homeland Security, Science and Technology Directorate, Office of University Programs



Thank you